

Joint UK-IGS EGGS Meeting London 08/Dec/2009

Basally Reinforced Platforms for Piled Embankments –

Two Case Studies:



A421 M1 Junction 13 to Bedford Improvements, UK Martin Curd, Scott Wilson Ltd



N210, Netherlands Hartmut Hangen, Huesker GmbH



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Piled Embankments - A Solution

What is the problem?

- Weak, compressible soils (i.e. alluvium, peat, normally consolidated clays and made ground)
- Leading to high settlements,
- Which may have a knock-on effect of long construction times (i.e. surcharge periods, staged construction) or unacceptable post construction settlements and differential settlements.



Available Solutions

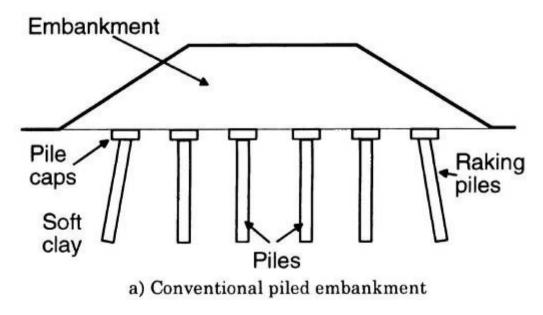
- Excavate and replace
- Staged construction; with and without the use of surcharge and vertical drains.
- Lightweight Fill
- Ground improvement;

Vibro stone columns, vibro concrete columns, soil mixing, dynamic compaction

- Piled reinforced concrete raft
- Piled embankment



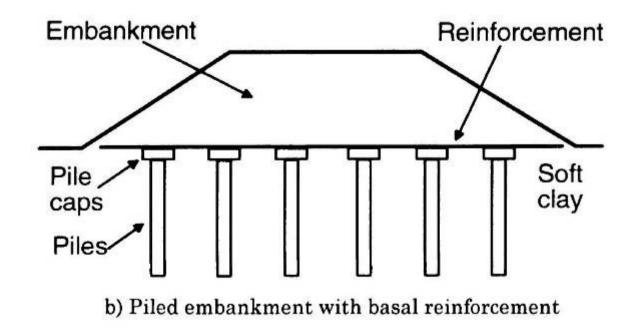
Relies solely on arching in embankment fill, can lead to punching failure where the piles punch through into the embankment fill. Consideration required to ensure the edge piles can take the lateral loads imposed by lateral thrust in the embankment, i.e. stronger piles or raking piles.



⁽Fig 66a from BS8006)



Relies on soil arching in the embankment and membrane action within the reinforcement. The reinforcement takes the lateral embankment thrust loads so raking piles not required.



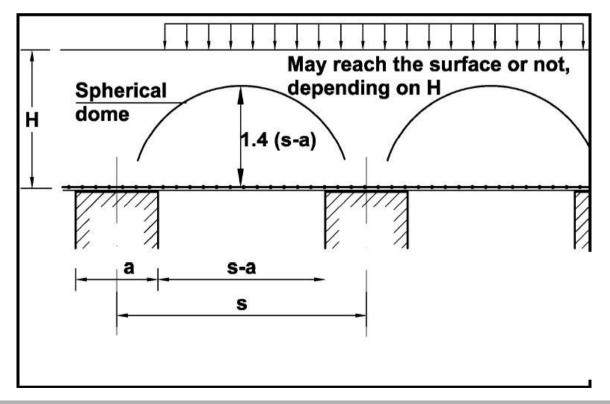
(Fig 66b from BS8006)



General Mechanism of Piled Embankments with Reinforcement

• Arching Mechanism

(as BS8006 1997)





General Mechanism of Piled Embankments with Reinforcement

• Membrane action mechanism

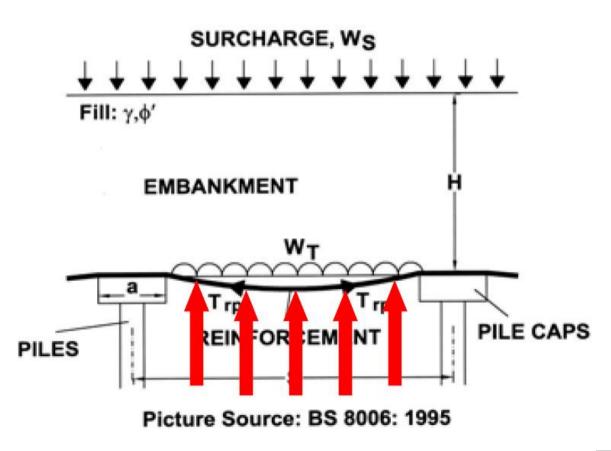


Photograph courtesy of Huesker Ltd



General Mechanism of Piled Embankments with Reinforcement

Counter pressure – subsoil support acting to resist the formation of hammocks in the reinforcement





A421 M1 Junction 13 to Bedford Improvements, Bedfordshire, UK



Photograph courtesy of BBCEL

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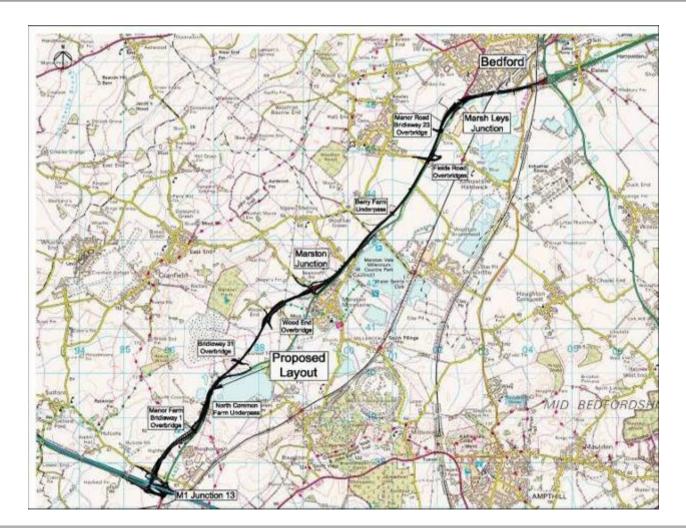


Stakeholders:

- Client: Highways Agency
- Contractor: Balfour Beatty Civil Engineering
- Engineer: Scott Wilson Ltd
- Reinforcement Supplier: Huesker Ltd
- Piling Contractor: Stent Foundations
- Earthworks Contractor: John Jones Excavation

A421 M1 Junction 13 to Bedford Improvements Scheme.





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Clay Pits: Historic Aerial Photography



Aerial photo from 1962



Brogborough Lake Clay Pit



Brogborough Clay Pits – Ground Conditions

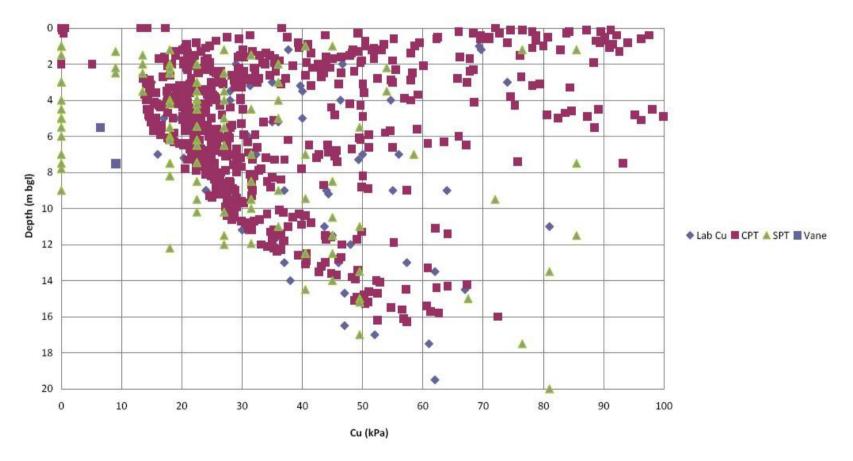


Made ground consisting of soft clay to depths up to 20m:

- Desiccated crust
- Soft clay, increasing in stiffness below around 8m depth
- Oxford Clay Formation

Brogborough Lake -Undrained Shear Strength

Brogborough Lake Made Ground Undrained Shear Strength







Geometry

- Embankment heights above the reinforcement between
 1.5m and 7m
- Clay fill embankment
- 1v in 3h side slopes
- (1v in 1h reinforced side slopes adjacent to the landfill to prevent the embankment encroaching onto the landfill).
- 275mm square precast driven piles with cast insitu 900mm diameter pile cap
- Piles at 1.75m, 2.0m and 2.5m centres

Design Aspects and its Realization in Practice



BS 8006 – limit strain / deformation

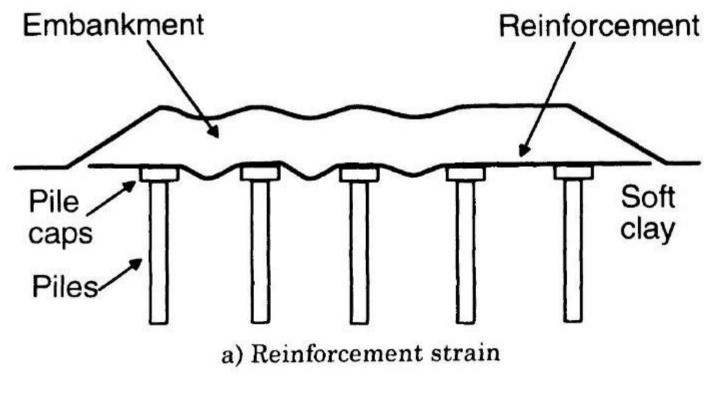


Fig 68a from BS8006

Low Embankment Heights: Maximum Design Strain 3%



- Multiple layers of high strength polyethylene reinforcement (strain at peak strength c. 10% - 12%)
- Single layer (in each direction) of high strength low strain Poly-Vinyl Alcohol (PVA) reinforcement (strain at peak strength c. 6%)
- Huesker Fortrac R-MP reinforcement used



Strain the reinforcement during construction in order to reduce post construction movements:

- Lay the reinforcement flat and even
- Adequate anchoring of reinforcement
- Allow reinforcement to form hammocks during construction – minimise counter pressure: use compressible fill.



Fill between pile caps needs to be:

- Sufficiently stiff to allow the reinforcement to be placed flat and without sagging
- Compressible enough to allow rapid deflection under load.

Reliance on the weak substrata to deform:

- Loads the substrata, inducing settlements (-ve SF on piles)
- Time for consolidation



- Peat has been used where locally available
- Absence of peat in Bedfordshire
- BBCEL proposed compost as an alternative
- Compost sourced from manufacturers of compost from garden waste
- Static load and compaction testing indicated compost had adequate compressibility



Compost Layer



Photograph courtesy of BBCEL

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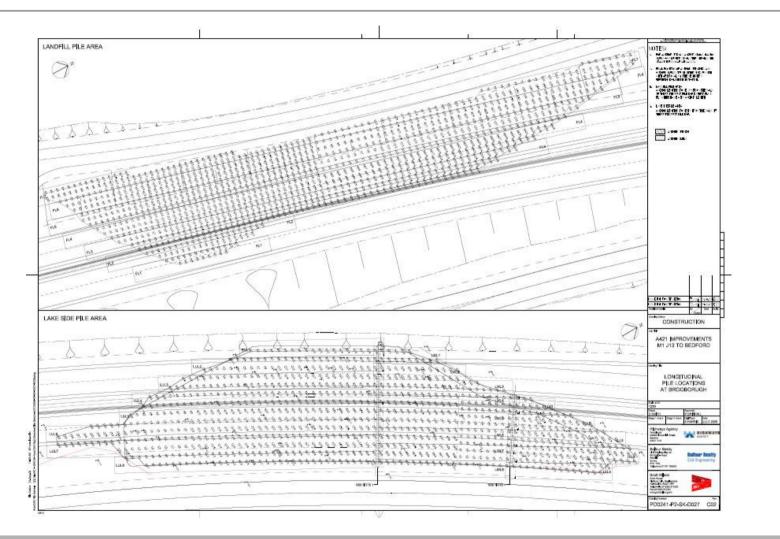


Construction

- Reinforcement ordered in bespoke lengths, each roll has its specific place in the construction
- 500mm of Class 6I/6J over the reinforcement
- Embankment made of Class 2 (site won Glacial Clay)
- Hi-visibility warning layer between the Class 6I/6J layer and the overlying Class 2 embankment



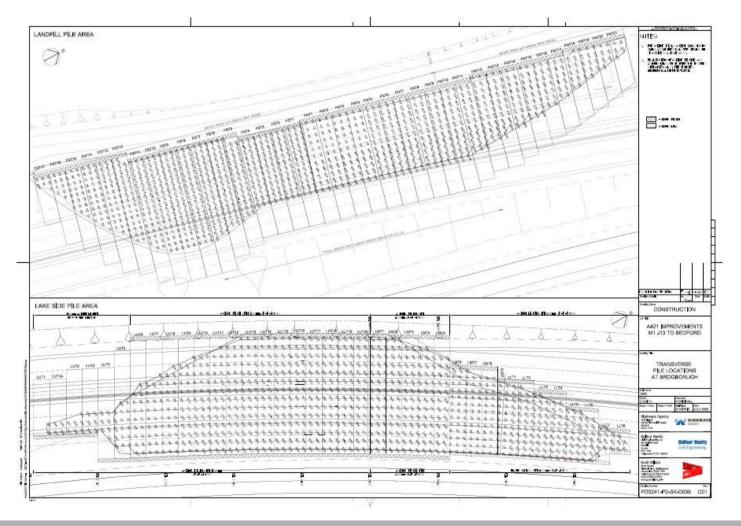
Reinforcement layout (1)



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Reinforcement layout (2)



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Construction - Driving Piles



Photograph courtesy of BBCEL

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Construction - Casting Pile Caps



Photograph courtesy of BBCEL

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Construction - Placing Compost Between Pile Caps



Photograph courtesy of BBCEL

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Construction - Excavator Tracks on Pile Caps



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Construction - Rolling Out Reinforcement



Photograph courtesy of BBCEL

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Construction - Anchor Trench



Photograph courtesy of BBCEL

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Construction - High-Visibility Warning Layer



Photograph courtesy of BBCEL

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Performance

- The embankment is still under construction
- Final piles were installed in mid November 2009.
- Instrumentation: Rod and plate settlement gauges above some of the pile caps, with the aim of monitoring the settlement of the pile group
- Construction is going well.